



Economically Sound Methods of Designing Water Supply and Distribution Systems

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Abstract: Is proved in article decisions on development of a water handling half-scientists only on the basis of an extensive and reliable data about technical condition of separate elements of these systems and parameters of the modes of their work, i.e. on data of redesigns researches.

Keywords: water taps, uneven pressure, internal water supply and sewerage of buildings, utilities, pressure instability, pump, tanks, pressure, network failure, internal water supply systems, frequency converter, water supply and distribution systems.

Reasonable decisions on the development of water supply systems can only be obtained on the basis of extensive and reliable data on the technical condition of individual elements of these systems and the parameters of their operating modes, i.e. based on pre-project survey data. However, in many cases it is not enough to use only the documentation available in operating organizations that characterizes the parameters of individual elements and equipment. This is due to the fact that during operation there is a continuous and regular change in the technical parameters of the structure, and the actual operating modes of the water supply and distribution systems (WDS) differ significantly from the calculated ones regulated by previously developed projects.

The collection of information on the actual state of the PRV systems is a rather complex technical task, the solution of which is not always possible by the forces of design organizations. At the same time, the work carried out by some organizations indicates that by obtaining extensive information about the water supply and distribution system (WDP), it is possible to obtain a significant economic effect and improve consumer provision. So the Academy of Public Utilities named after K.D. Pamfilov [1, 3] developed a set of methodological techniques, in which decisions on managing the development and reconstruction of WDP systems are prepared on the basis of special surveys of WDP systems and equivalent calculation schemes.

The first stage in the construction of equivalent design schemes (models), which, in terms of the main parameters (pressure at network nodes, pressure at pumping stations, water flow through water conduits and the most important network lines, hydraulic resistance of pipeline sections) with sufficient accuracy, corresponds to real conditions, consists in determining water consumption values for various subscribers, in the analysis of a complete map of the water supply network and its simplification. The difficulty of simplifying the complete network diagram is due to the fact that the hydraulic calculation of complex ring networks, even with the use of modern computer technology, is not an easy task. In practice, the number of rings in networks that can be calculated using widely used computers does not exceed 200-300, while real networks of large cities can have 1500-2000 rings. Therefore, it is recommended to leave in the design scheme only the main trunk lines with the highest load, excluding almost the entire distribution network and dead-end lines. In many cases, the number of design scheme elements is reduced by combining several network lines into one scheme element with equivalent hydraulic resistance.



Data on the average water consumption in the WDP system is obtained from the materials of subscriber departments or control measurements. This requires special care, because due to the imperfection of the instruments for recording the amount of water consumed, data on water consumption do not have high accuracy. It is recommended to experimentally determine the water consumption at pumping stations and for the largest consumers (with a flow rate of more than 1-2% of the total water consumption). Data on water consumption is systematized and applied to the calculation scheme in the form of nodal selections. One of the most important stages of work on constructing an equivalent model of the WDP system is carrying out field surveys, determining the actual hydraulic resistance of existing pipelines and the flow-pressure characteristics of pumping units [2].

Manometric survey data during characteristic periods of relatively stable water consumption (from 11 am to 3 pm and from 1 am to 5 am on weekdays) is the basis for assessing the equivalence of the design scheme of the WDP system to real operating conditions. Gauge survey, i.e. pressure measurement, is carried out in a sufficiently large number of nodes (main intersections of highways), the number of which should be at least 70% of the total number of network nodes. For large networks, measurements are carried out over several days, while the stability of water consumption regimes must be confirmed by the data of the dispatching service, which should prevent a sharp change in the operating modes of the main structures (pumping stations, tanks, etc.) during the period of manometric survey.

Manometric survey data are the basis for constructing the movement of water. Already a preliminary analysis of the manometric survey data makes it possible to identify sections of the network with abnormally high pressure losses (due to closed valves, blockages, freezing, etc.), and the elimination of these anomalies leads to an improvement in the water supply of some local areas of the water supply network.

The most important type of natural testing is the work to determine the actual hydraulic resistance of pipelines. It is known that as a result of corrosion and hydraulic fouling, the operating conditions of long-term pipelines can differ significantly from the conditions of new pipes. In addition, poor-quality construction of pipelines also leads to an increase in hydraulic resistance. The pattern of fouling on the inner surface of pipelines can be analyzed by inspection, but obtaining quantitative characteristics is possible only through experimental studies. The most reliable and practically accessible method of such studies is the determination of the hydraulic resistance of pipelines according to the measurement data of pressure tubes of the velocities of water at various points in the section, including on the axis of the pipe.

As a result of a number of studies, dependencies were obtained for calculating the values of the coefficients of hydraulic resistance according to the measured speeds [3]. These studies are the most time-consuming and cannot be performed on all sections of the network. Nevertheless, it is recommended to cover sections of pipelines made of different materials, different years of construction, different diameters in order to fairly reasonably extrapolate the results of a limited number of measurements to the entire network.

Other methods for measuring hydraulic resistance are also used, which are based on determining the pressure difference in the initial and end sections of pipelines, the water flow rates for which are determined using water meters.

During field surveys of WDP systems, it is recommended to determine the flow-pressure characteristics of pumping units, since during operation, due to wear and tear of equipment, these characteristics are quite different from the passport data. It should be noted that it is the stage of field surveys that is the most difficult for design organizations, but only the mandatory performance of the work of this stage by specialized survey departments of design institutes,



commissioning organizations or operational services can guarantee the receipt of a sufficient amount of initial information about the operating conditions of the WDP system.

The absence of this information will lead to the fact that the decision made on the development of the system will not correspond to the actual situation and will not lead to a significant improvement in the water supply of consumers, will not provide the ability to connect new subscribers to the system and normal operation of the system for the entire accepted estimated period of the project. Experimentally determined coefficients of hydraulic resistance of pipelines of the WDP system make it possible to calculate correction factors for tables of these resistance values. However, automatic extrapolation of such data to all elements of the calculation scheme is possible, since even for pipes of the same laying year and made of the same material, the hydraulic resistance coefficients can actually have significant differences. In addition, the elements of the design scheme, obtained by combining several sections connected in parallel or in series, have an equivalent resistance, which should be corrected very carefully according to the measurement data. Therefore, the next stage of the described work is to identify the design scheme, i.e. in its correction in such a way that the values of piezometric pressures calculated by hydraulic calculations according to this scheme at the network nodes coincide quite accurately with the data of field measurements. Here, a step-by-step correction of the parameters of the design scheme is used, which is carried out until the difference between the measured and calculated piezometers is ≤ 1 m. Usually, it is enough to carry out 5–6 correction stages at which the coefficients of hydraulic resistance of the lines of the design scheme are multiplied by the value

$$K_i = \Delta h_{i \text{ fakto}} / \Delta h_{i \text{ dif}}$$

where K_i - correction factor for the i -th section of the network;

$\Delta h_{i \text{ fakto}}$ - difference between the heads measured at the ends of the i -th section;

$\Delta h_{i \text{ dif}}$ - difference of design pressures at the ends of the i -th section.

In the process of correcting the parameters of the design scheme, the measurement of the values of the hydraulic resistance coefficients serve as restrictions on the values K_i (greatest deviation K_i from the measured values should not exceed $\pm 50\%$) for those sections of the design scheme on which the measurement was not carried out.

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